

- Column (1) - Design flow to be conveyed by pipe segment.
- Column (2) - Length of pipe segment.
- Column (3) - Pipe Size; indicate pipe diameter or span x rise.
- Column (4) - Manning's "n" value.
- Column (5) - Outlet Elevation of pipe segment.
- Column (6) - Inlet Elevation of pipe segment.
- Column (7) - Barrel Area; this is the full cross-sectional area of the pipe.
- Column (8) - Barrel Velocity; this is the full velocity in the pipe as determined by:

$$V = Q/A \text{ or } \text{Col.}(8) = \text{Col.}(1) / \text{Col.}(7)$$
- Column (9) - Barrel Velocity Head = $V^2/2g$ or $(\text{Col.}(8))^2/2g$
 where $g = 32.2 \text{ ft/sec}^2$ (acceleration due to gravity)
- Column (10) - Tailwater (TW) Elevation; this is the water surface elevation at the outlet of the pipe segment. If the pipe's outlet is not submerged by the TW and the TW depth is less than $(D+d_c)/2$, set TW equal to $(D+d_c)/2$ to keep the analysis simple and still obtain reasonable results (D = pipe barrel height and d_c = critical depth, both in feet. See Figure F. 14 for determination of d_c).
- Column (11) - Friction Loss = $S_f \times L$ [or $S_f \times \text{Col.}(2)$]
 where S_f is the friction slope or head loss per linear foot of pipe as determined by Manning's equation expressed in the form:

$$S_f = (nV)^2/2.22 R^{4/3}$$
- Column (12) - Hydraulic Grade Line (HGL) Elevation just inside the entrance of the pipe barrel; this is determined by adding the friction loss to the TW elevation:

$$\text{Col.}(12) = \text{Col.}(11) + \text{Col.}(10)$$

 If this elevation falls below the pipe's inlet crown, it no longer represents the true HGL when computed in this manner. The true HGL will fall somewhere between the pipe's crown and either normal flow depth or critical flow depth, whichever is greater. To keep the analysis simple and still obtain reasonable results (i.e., erring on the conservative side), set the HGL elevation equal to the crown elevation.
- Column (13) - Entrance Head Loss = $K_e \times V^2/2g$ [or $K_e \times \text{Col.}(9)$]
 where K_e = Entrance Loss Coefficient (from Table F. 4). This is the head lost due to flow contractions at the pipe entrance.
- Column (14) - Exit Head Loss = $1.0 \times V^2/2g$ or $1.0 \times \text{Col.}(9)$
 This is the velocity head lost or transferred downstream.
- Column (15) - Outlet Control Elevation = $\text{Col.}(12) + \text{Col.}(13) + \text{Col.}(14)$
 This is the maximum headwater elevation assuming the pipe's barrel and inlet/outlet characteristics are controlling capacity. It does not include structure losses or approach velocity considerations.
- Column (16) - Inlet Control Elevation (see Appendix F for computation of inlet control on culverts); this is the maximum headwater elevation assuming the pipe's inlet is controlling capacity. It does not include structure losses or approach velocity considerations.
- Column (17) - Approach Velocity Head; this is the amount of head/energy being supplied by the discharge from an upstream pipe or channel section, which serves to reduce the headwater elevation. If the discharge is from a pipe, the approach velocity head is equal to the barrel velocity head computed for the upstream pipe. If the upstream pipe outlet is significantly higher in elevation (as in a drop man hole) or lower in elevation such that its discharge energy would be dissipated, an approach velocity head of zero should be assumed.
- Column (18) - Bend Head Loss = $K_b \times V^2/2g$ [or $K_b \times \text{Col.}(17)$]
 where K_b = Bend Loss Coefficient (from Figure F. 7). This is the loss of head/energy required to change direction of flow in an access structure.
- Column (19) - Junction Head Loss. This is the loss in head/energy that results from the turbulence created when two or more streams are merged into one within the access structure. Figure F. 8 may be used to determine this loss, or it may be computed using the following equations derived from Figure F. 8:

$$\text{Junction Head Loss} = K_j \times V^2/2g \text{ [or } K_j \times \text{Col.}(17)]$$

 where K_j is the Junction Loss Coefficient determined by:

$$K_j = (Q_2/Q_1)(1.18 + 0.63(Q_2/Q_1))$$
- Column (20) - Headwater (HW) Elevation; this is determined by combining the energy heads in Columns 17, 18, and 19 with the highest control elevation in either Column 15 or 16, as follows:

$$\text{Col.}(20) = \text{Col.}(15 \text{ or } 16) - \text{Col.}(17) + \text{Col.}(18) + \text{Col.}(19)$$



Figure F.5. Backwater Calculation Sheet Notes